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 Design: | Piece#8-top anchored- Oct 3, 2025  
 Fastening point:

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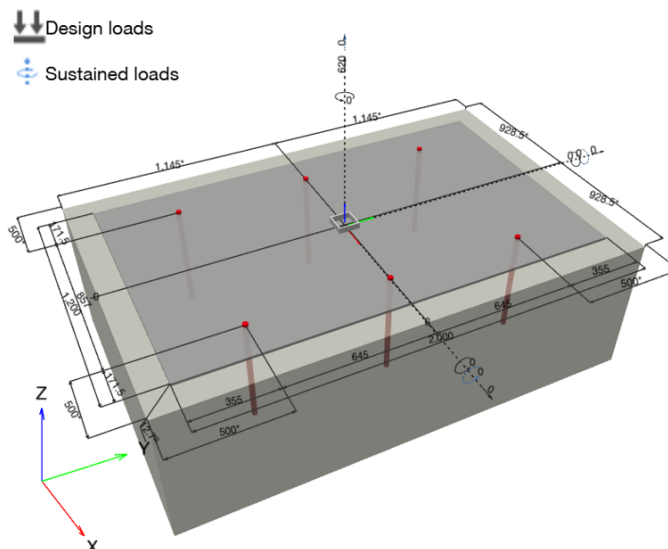
**Specifier's comments:**

**1 Input data**

<b>Anchor type and diameter:</b>	<b>HIT-RE 500 V3 + HAS-B-105 (ASTM F1554 Gr.105) 1</b>	
Item number:	not available (element) / 2123401 HIT-RE 500 V3 (adhesive)	
Specification text:	Hilti $\varnothing$ 1 in HIT-RE 500 V3 + HAS-B-105 (ASTM F1554 Gr.105) with 500 mm nominal embedment depth per ICC-ES ESR-3814 , SAFEset - automatic cleaning installation per MPII,	
Effective embedment depth:	$h_{ef,act} = 500.0$ mm ( $h_{ef,limit} = -$ mm)	
Material:	ASTM F1554 Grade 105	
Evaluation Service Report:	ESR-3814	
Issued   Valid:	-   -	
Proof:	Design Method CSA A23.3-14 / Chem.	
Shear edge breakout verification:	Row closest to edge (Case 3 only from CSA A23.3-14 Fig. D.13)	
Stand-off installation:	$e_b = 0.0$ mm (no stand-off); $t = 12.7$ mm	
Anchor plate <sup>R</sup> :	$l_x \times l_y \times t = 1,200.0$ mm x $2,000.0$ mm x $12.7$ mm; (Recommended plate thickness: not calculated)	
Profile:	Square HSS (AISC), HSS4X4X.25; (L x W x T) = $101.6$ mm x $101.6$ mm x $6.3$ mm	
Base material:	cracked concrete, $f'_c = 34.47$ N/mm <sup>2</sup> ; $h = 3,000.0$ mm, Temp. short/long: 20/20 °C	
<b>Installation:</b>	<b>automatic cleaned drilled hole, Installation condition: Dry</b>	
Reinforcement:	tension: condition B, shear: condition B; no supplemental splitting reinforcement present edge reinforcement: none or < 15M bar	

<sup>R</sup> - The anchor calculation is based on a rigid anchor plate assumption.

**Geometry [mm] & Loading [kN, kNm]**



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**1.1 Design results**

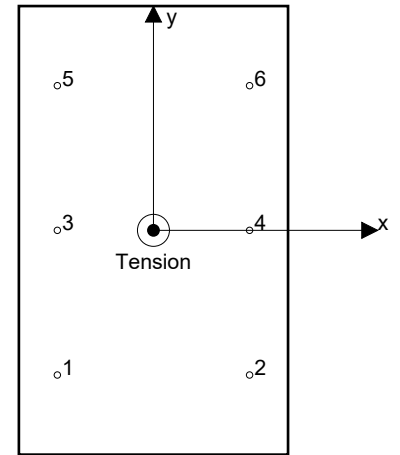
Case	Description	Forces [kN] / Moments [kNm]	Seismic	Max. Util. Anchor [%]
1	Combination 1	N = 620.000; V <sub>x</sub> = 0.000; V <sub>y</sub> = 0.000; M <sub>x</sub> = 0.000; M <sub>y</sub> = 0.000; M <sub>z</sub> = 0.000; N <sub>sus</sub> = 0.000; M <sub>x,sus</sub> = 0.000; M <sub>y,sus</sub> = 0.000;	no	89

**2 Load case/Resulting anchor forces**

**Anchor reactions [kN]**

Tension force: (+Tension, -Compression)

Anchor	Tension force	Shear force	Shear force x	Shear force y
1	103.333	0.000	0.000	0.000
2	103.333	0.000	0.000	0.000
3	103.333	0.000	0.000	0.000
4	103.333	0.000	0.000	0.000
5	103.333	0.000	0.000	0.000
6	103.333	0.000	0.000	0.000



Max. concrete compressive strain: - [%]  
 Max. concrete compressive stress: - [N/mm<sup>2</sup>]  
 Resulting tension force in (x/y)=(0.0/0.0): 620.000 [kN]  
 Resulting compression force in (x/y)=(-/-): 0.000 [kN]

Anchor forces are calculated based on the assumption of a rigid anchor plate.

**3 Tension load**

	Load N <sub>f</sub> [kN]	Capacity N <sub>r</sub> [kN]	Utilization β <sub>N</sub> = N <sub>f</sub> /N <sub>r</sub>	Status
Steel Strength*	103.333	229.022	46	OK
Bond Strength**	620.000	1,351.259	46	OK
Sustained Tension Load Bond Strength*	N/A	N/A	N/A	N/A
Concrete Breakout Failure**	620.000	701.264	89	OK

\* highest loaded anchor    \*\*anchor group (anchors in tension)

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**3.1 Steel Strength**

$$N_{s,uta} = A_{se} f_{uta} = \text{ESR value} \quad \text{refer to ICC-ES ESR-3814}$$

$$N_{sar} = A_{se} \phi_s f_{uta} R \quad \text{CSA A23.3-14 Eq. D.2}$$

$$N_{sar} \geq N_{fa} \quad \text{CSA A23.3-14 Table D.1}$$

**Variables**

n	$A_{se,N}$ [mm <sup>2</sup> ]	$f_{uta}$ [N/mm <sup>2</sup> ]
1	391	861.84

**Calculations**

$$N_{s,uta} \text{ [kN]}$$

336.797

**Results**

$N_{s,uta}$ [kN]	$\phi_s$	R	$N_{sar}$ [kN]	$N_{fa}$ [kN]
336.797	0.850	0.800	229.022	103.333

**3.2 Bond Strength**

$$N_{agr} = \left( \frac{A_{Na}}{A_{Na0}} \right) \Psi_{ec1,Na} \Psi_{ec2,Na} \Psi_{ed,Na} \Psi_{cp,Na} N_{bar} \quad \text{CSA A23.3-14 Eq. D.21}$$

$$N_{agr} \geq N_{fa,g} \quad \text{CSA A23.3-14 Table D.1}$$

$$A_{Na} = \text{see CSA A23.3-14, Part D.6.5.1, Fig. D.11}$$

$$A_{Na0} = (2 c_{Na})^2 \quad \text{CSA A23.3-14 Eq. D.22}$$

$$c_{Na} = 10 d_a \sqrt{\frac{\tau_{uncr}}{7.60}} \quad \text{CSA A23.3-14 Eq. D.23}$$

$$\Psi_{ec,Na} = \left( \frac{1}{1 + \frac{e_N}{c_{Na}}} \right) \leq 1.0 \quad \text{CSA A23.3-14 Eq. D.25}$$

$$\Psi_{ed,Na} = 0.7 + 0.3 \left( \frac{c_{a,min}}{c_{Na}} \right) \leq 1.0 \quad \text{CSA A23.3-14 Eq. D.27}$$

$$\Psi_{cp,Na} = \text{MAX} \left( \frac{c_{a,min}}{c_{ac}}, \frac{c_{Na}}{c_{ac}} \right) \leq 1.0 \quad \text{CSA A23.3-14 Eq. D.29}$$

$$N_{bar} = \lambda_a \phi_c \tau_r \pi d_a h_{ef} R \quad \text{CSA A23.3-14 Eq. D.24}$$

**Variables**

$\tau_{uncr}$ [N/mm <sup>2</sup> ]	$d_a$ [mm]	$h_{ef}$ [mm]	$c_{a,min}$ [mm]	$\alpha_{overhead}$	$\tau_{cr}$ [N/mm <sup>2</sup> ]
16.07	25.4	500.0	500.0	1.000	9.49
$e_{1,N}$ [mm]	$e_{2,N}$ [mm]	$c_{ac}$ [mm]	$\lambda_a$		
0.0	0.0	925.3	1.000		

**Calculations**

$c_{Na}$ [mm]	$A_{Na}$ [mm <sup>2</sup> ]	$A_{Na0}$ [mm <sup>2</sup> ]	$\Psi_{ed,Na}$
369.4	2,997,249	545,691	1.000

$\Psi_{ec1,Na}$	$\Psi_{ec2,Na}$	$\Psi_{cp,Na}$
1.000	1.000	1.000

**Results**

$\phi_c$	R	$N_{bar}$ [kN]	$N_{agr}$ [kN]	$N_{fa,g}$ [kN]
0.650	1.000	246.015	1,351.259	620.000

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**3.3 Concrete Breakout Failure**

$$N_{cbgr} = \left( \frac{A_{Nc}}{A_{Nc0}} \right) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_{br} \quad \text{CSA A23.3-14 Eq. D.4}$$

$$N_{cbgr} \geq N_{fa,g} \quad \text{CSA A23.3-14 Table D.1}$$

$$A_{Nc} \text{ see CSA A23.3-14, Part D.6.2.1, Fig. D.7}$$

$$A_{Nc0} = 9 h_{ef}^2 \quad \text{CSA A23.3-14 Eq. D.5}$$

$$\Psi_{ec,N} = \left( \frac{1}{1 + \frac{2 e_N}{3 h_{ef}}} \right) \leq 1.0 \quad \text{CSA A23.3-14 Eq. D.8}$$

$$\Psi_{ed,N} = 0.7 + 0.3 \left( \frac{c_{a,min}}{1.5 h_{ef}} \right) \leq 1.0 \quad \text{CSA A23.3-14 Eq. D.11}$$

$$\Psi_{cp,N} = \text{MAX} \left( \frac{c_{a,min}}{c_{ac}}, \frac{1.5 h_{ef}}{c_{ac}} \right) \leq 1.0 \quad \text{CSA A23.3-14 Eq. D.13}$$

$$N_{br} = k_c \phi_c \lambda_a \sqrt{f_c} h_{ef}^{1.5} R \quad \text{CSA A23.3-14 Eq. D.6}$$

**Variables**

$h_{ef}$ [mm]	$e'_{1,N}$ [mm]	$e'_{2,N}$ [mm]	$c_{a,min}$ [mm]	$\Psi_{c,N}$
333.3	0.0	0.0	500.0	1.000
$c_{ac}$ [mm]	$k_c$	$\lambda_a$	$f_c$ [N/mm <sup>2</sup> ]	
925.3	7.1	1.000	34.47	

**Calculations**

$A_{Nc}$ [mm <sup>2</sup> ]	$A_{Nc0}$ [mm <sup>2</sup> ]	$\Psi_{ec1,N}$	$\Psi_{ec2,N}$	$\Psi_{ed,N}$	$\Psi_{cp,N}$
4,252,530	1,000,000	1.000	1.000	1.000	1.000

**Results**

$\phi_c$	R	$N_{br}$ [kN]	$N_{cbgr}$ [kN]	$N_{fa,g}$ [kN]
0.650	1.000	164.905	701.264	620.000



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#### 4 Shear load

	Load $V_f$ [kN]	Capacity $V_r$ [kN]	Utilization $\beta_V = V_f/V_r$	Status
Steel Strength*	N/A	N/A	N/A	N/A
Steel failure (with lever arm)*	N/A	N/A	N/A	N/A
Pryout Strength (Bond Strength controls)*	N/A	N/A	N/A	N/A
Concrete edge failure in direction **	N/A	N/A	N/A	N/A

\* highest loaded anchor    \*\*anchor group (relevant anchors)

#### 5 Warnings

- The anchor design methods in PROFIS Engineering require rigid anchor plates per current regulations (EN1992-4, AS5216, etc.). This means load re-distribution on the anchors due to elastic deformations of the anchor plate are not considered - the anchor plate is assumed to be sufficiently stiff, in order not to be deformed when subjected to the design loading. PROFIS Engineering calculates the minimum required anchor plate thickness with FEM to limit the stress of the anchor plate based on the assumptions explained above. The proof if the rigid anchor plate assumption is valid is not carried out by PROFIS Engineering. Input data and results must be checked for agreement with the existing conditions and for plausibility!
- The equations presented in this report are based on metric units. When inputs are displayed in imperial units, the user should be aware that the equations remain in their metric format.
- Condition A applies when supplementary reinforcement is used. The R factor is increased for non-steel Design Strengths except Pullout Strength and Pryout strength. Condition B applies when supplementary reinforcement is not used and for Pullout Strength and Pryout Strength. Refer to your local standard.
- Checking the transfer of loads into the base material is required in accordance with CSA A23.3!
- Installation of Hilti adhesive anchor systems shall be performed by personnel trained to install Hilti adhesive anchors. CSA A23.3-14 Annex D, Clause D.10.1

**Fastening meets the design criteria!**

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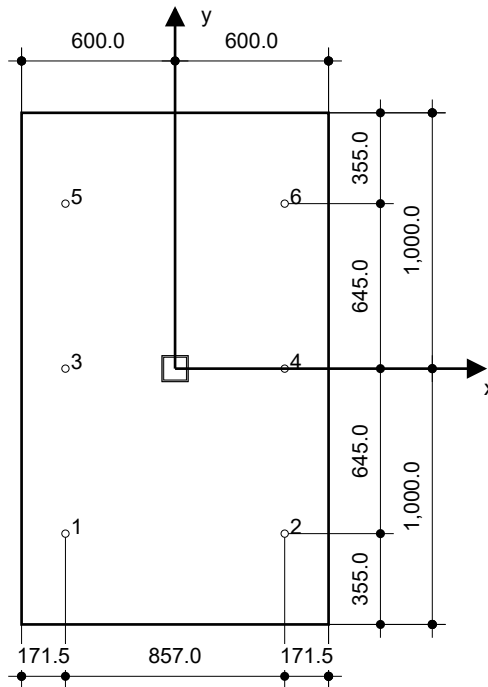
**6 Installation data**

Profile: Square HSS (AISC), HSS4X4X.25; (L x W x T) = 101.6 mm x 101.6 mm x 6.3 mm	Anchor type and diameter: HIT-RE 500 V3 + HAS-B-105 (ASTM F1554 Gr.105) 1
Hole diameter in the fixture: $d_f = 28.6$ mm	Item number: not available (element) / 2123401 HIT-RE 500 V3 (adhesive)
Plate thickness (input): 12.7 mm	Maximum installation torque: 203 Nm
Recommended plate thickness: not calculated	Hole diameter in the base material: 28.6 mm
Drilling method: SafeSet - automatic cleaning	Hole depth in the base material: 500.0 mm
Cleaning: Automatically performed while drilling	Minimum thickness of the base material: 557.1 mm

Hilti  $\varnothing 1$  in HIT-RE 500 V3 + HAS-B-105 (ASTM F1554 Gr.105) with 500 mm nominal embedment depth per ICC-ES ESR-3814 , SAFEset - automatic cleaning installation per MPII

**6.1 Recommended accessories**

Drilling	Cleaning	Setting
<ul style="list-style-type: none"> <li>Suitable Rotary Hammer</li> <li>Properly sized drill bit for SAFEset - automatic cleaning (TE-CD / TE-YD)</li> <li>Vacuum cleaner</li> </ul>	<ul style="list-style-type: none"> <li>No accessory required</li> </ul>	<ul style="list-style-type: none"> <li>Dispenser including cassette and mixer</li> <li>Torque wrench</li> </ul>



**Coordinates Anchor [mm]**

Anchor	x	y	C <sub>-x</sub>	C <sub>+x</sub>	C <sub>-y</sub>	C <sub>+y</sub>	Anchor	x	y	C <sub>-x</sub>	C <sub>+x</sub>	C <sub>-y</sub>	C <sub>+y</sub>
1	-428.5	-645.0	500.0	1,357.0	500.0	1,790.0	4	428.5	0.0	1,357.0	500.0	1,145.0	1,145.0
2	428.5	-645.0	1,357.0	500.0	500.0	1,790.0	5	-428.5	645.0	500.0	1,357.0	1,790.0	500.0
3	-428.5	0.0	500.0	1,357.0	1,145.0	1,145.0	6	428.5	645.0	1,357.0	500.0	1,790.0	500.0

Input data and results must be checked for conformity with the existing conditions and for plausibility!  
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## 7 Remarks; Your Cooperation Duties

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