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Design: TEST Date: 22/05/2020
Fastening Point: Base plate

Specifier's comments:

1 Input data

Anchor type and size: HIT-HY 200-A + HAS-U A4 M20

Return period (service life in years): 50

Item number: 2223929 HAS-U A4 M20x350 (insert) / 2022696

HIT-HY 200-A (mortar)

Effective embedment depth: $h_{ef,act} = 200.0 \text{ mm } (h_{ef,limit} = - \text{ mm})$

Material: A4

Approval No.: ETA 11/0493

Issued I Valid: 30/08/2019 | -

Proof: Design Method EN 1992-4, Chemical

Stand-off installation: without clamping (anchor); restraint level (baseplate): 2.00; e_b = 35.0 mm; t = 35.0 mm

Hilti Grout: , multipurpose, f_{c,Grout} = 30.00 N/mm²

Baseplate^R: $I_x \times I_y \times t = 400.0 \text{ mm} \times 400.0 \text{ mm} \times 35.0 \text{ mm}$; (Recommended plate thickness: not calculated)

Profile: Square hollow, 200 x 200 x 10,0; (L x W x T) = 200.0 mm x 200.0 mm x 10.0 mm

Base material: cracked concrete, C40/50, $f_{c.cvl}$ = 40.00 N/mm²; h = 365.0 mm, Temp. short/long: 0/0 °C

Installation: hammer drilled hole, Installation condition: Dry

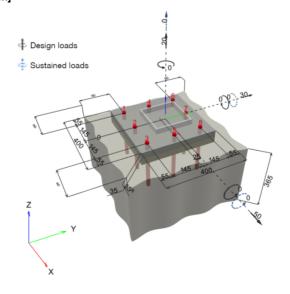
Reinforcement: No reinforcement or Reinforcement spacing >= 150 mm (any Ø) or >= 100 mm (Ø <= 10 mm)

no longitudinal edge reinforcement

Application also possible with HVU2 + HAS-U A4 M20 under the selected boundary conditions. More information in section Alternative fastening data of this report.

^R - The anchor calculation is based on a rigid baseplate assumption.

Geometry [mm] & Loading [kN, kNm]







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1.1 Load combination

Case	Description	Forces [kN] / Moments [kNm]	Seismic	Fire	Max. Util. Anchor [%]
1	Combination 1	N = 20.000; $V_x = 50.000$; $V_y = 30.000$; $M_v = 0.000$; $M_v = 0.000$; $M_z = 0.000$;	no	no	92
		$N_{sus} = 0.000; M_{x,sus} = 0.000; M_{y,sus} = 0.000;$			

2 Load case/Resulting anchor forces

Anchor reactions [kN]

Tension force: (+Tension, -Compression)

Anchor	Tension force	Shear force	Shear force x	Shear force y
1	1.925	7.111	5.532	4.468
2	1.925	6.683	5.532	3.750
3	1.925	6.308	5.532	3.032
4	2.500	7.683	6.250	4.468
5	2.500	6.946	6.250	3.032
6	3.075	8.278	6.968	4.468
7	3.075	7.913	6.968	3.750
8	3.075	7.599	6.968	3.032

 $\begin{array}{lll} \text{max. concrete compressive strain:} & - \left[\%\right] \\ \text{max. concrete compressive stress:} & - \left[\text{N/mm}^2\right] \\ \text{resulting tension force in (x/y)=(0.0/25.0):} & 20.000 \left[\text{kN}\right] \\ \text{resulting compression force in (x/y)=(0.0/0.0):} & 0.000 \left[\text{kN}\right] \end{array}$

Anchor forces are calculated based on the assumption of a rigid baseplate.



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3 Tension load (EN 1992-4, Section 7.2.1)

	Load [kN]	Capacity [kN]	Utilization β_N [%]	Status
Steel failure*	3.075	91.711	4	OK
Combined pullout-concrete cone failure**	20.000	190.094	11	OK
Concrete Breakout failure**	20.000	186.505	11	OK
Splitting failure**	N/A	N/A	N/A	N/A

^{*} highest loaded anchor **anchor group (anchors in tension)

3.1 Steel failure

$$N_{Ed} \le N_{Rd,s} = \frac{N_{Rk,s}}{\gamma_{M,s}}$$
 EN 1992-4, Table 7.1

N _{Rk,s} [kN]	$\gamma_{M,s}$	$N_{Rd,s}$ [kN]	N _{Ed} [kN]
171.500	1.870	91.711	3.075

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3.2 Combined pullout-concrete cone failure

$N_{Ed} \leq N_{Rd}$	$_{p} = \frac{N_{Rk,p}}{\gamma_{M,p}}$	EN 1992-4, Table 7.1
$N_{Rk,p}$	$= N_{Rk,p}^{0} \cdot \frac{A_{p,N}}{A_{p,N}^{0}} \cdot \psi_{g,Np} \cdot \psi_{s,Np} \cdot \psi_{re,N} \cdot \psi_{ec1,Np} \cdot \psi_{ec2,Np}$	EN 1992-4, Eq. (7.13)
$N_{Rk,p}^0$ Ψ_{sus}	$= \psi_{sus} \cdot \tau_{Rk} \cdot \pi \cdot d \cdot h_{ef}$ $= 1$	EN 1992-4, Eq. (7.14) EN 1992-4, Eq. (7.14a)
S _{cr,Np}	$= 7.3 \cdot d \cdot \sqrt{\psi_{sus} \cdot \tau_{Rk}} \le 3 \cdot h_{ef}$	EN 1992-4, Eq. (7.15)
$\psi_{g,Np}$	$= \psi_{g,Np}^{0} - \left(\frac{s}{s_{cr,Np}}\right)^{0.5} \cdot \left(\psi_{g,Np}^{0} - 1\right) \ge 1.00$	EN 1992-4, Eq. (7.17)
$\psi_{g,Np}^{0}$	$= \sqrt{n} - (\sqrt{n} - 1) \cdot \left(\frac{\tau_{Rk}}{\tau_{Rk,c}}\right)^{1,5} \ge 1.00$	EN 1992-4, Eq. (7.18)
$\tau_{\text{Rk,c}}$	$= \frac{k_3}{\pi \cdot d} \cdot \sqrt{h_{ef} \cdot f_{ck}}$	EN 1992-4, Eq. (7.19)
$\psi_{\text{ s,Np}}$	$= 0.7 + 0.3 \cdot \frac{c}{c_{cr,Np}} \le 1.00$	EN 1992-4, Eq. (7.20)
$\Psi_{\text{ ec1,Np}}$	$= \frac{1}{1 + \left(\frac{2 \cdot e_{c1,N}}{s_{cr,Np}}\right)} \le 1.00$	EN 1992-4, Eq. (7.21)
$\Psi_{\text{ ec2,Np}}$	$= \frac{1}{1 + \left(\frac{2 \cdot e_{c2,N}}{s_{cr,Np}}\right)} \le 1.00$	EN 1992-4, Eq. (7.21)

$A_{p,N}$ [mm ²]	$A_{p,N}^0$ [mm ²]	$\tau_{Rk,ucr,20}$ [N/mm ²]	s _{cr,Np} [mm]	c _{cr,Np} [mm]	c _{min} [mm]	f _{c,cyl} [N/mm ²]
792,100	360,000	18.00	600.0	300.0	∞	40.00
Ψ c	$\tau_{Rk,cr} [N/mm^2]$	k ₃	τ _{Rk,c} [N/mm ²]	$\psi^0_{g,Np}$	$\psi_{g,Np}$	_
1.079	9.17	7.700	10.96	1.429	1.218	
e _{c1,N} [mm]	$\Psi_{\text{ec1,Np}}$	e _{c2,N} [mm]	$\Psi_{\text{ec2,Np}}$	$\psi_{\text{s,Np}}$	$\psi_{\text{re},\text{Np}}$	_
0.0	1.000	25.0	0.923	1.000	1.000	
$_{ ext{sus}}^{ ext{0}}$	α_{sus}	$\Psi_{\sf sus}$				
0.740	0.000	1.000				
$N_{Rk,p}^0$ [kN]	N _{Rk,p} [kN]	$\gamma_{M,p}$	N _{Rd,p} [kN]	N _{Ed} [kN]	_	
115.277	285.141	1.500	190.094	20.000		

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3.3 Concrete Breakout failure

$N_{Ed} \le N_{Rd}$	'M.c	EN 1992-4, Table 7.1
$N_{Rk,c}$	$= N_{Rk,c}^{0} \cdot \frac{A_{c,N}}{A_{c,N}^{0}} \cdot \psi_{s,N} \cdot \psi_{re,N} \cdot \psi_{ec1,N} \cdot \psi_{ec2,N} \cdot \psi_{M,N}$	EN 1992-4, Eq. (7.1)
$N_{Rk,c}^0$ $A_{c,N}^0$	$= k_1 \cdot \sqrt{f_{ck}} \cdot h_{ef}^{1,5}$	EN 1992-4, Eq. (7.2)
$A_{c,N}^0$	$= s_{cr,N} \cdot s_{cr,N}$	EN 1992-4, Eq. (7.3)
$\Psi_{s,N}$	$= 0.7 + 0.3 \cdot \frac{c}{c_{cr,N}} \le 1.00$	EN 1992-4, Eq. (7.4)
$\psi_{\text{ ec1,N}}$	$= \frac{1}{1 + \left(\frac{2 \cdot e_{N,1}}{s_{cr,N}}\right)} \le 1.00$	EN 1992-4, Eq. (7.6)
$\psi_{\text{ ec2,N}}$	$= \frac{1}{1 + \left(\frac{2 \cdot e_{N,2}}{s_{cr,N}}\right)} \le 1.00$	EN 1992-4, Eq. (7.6)
$\psi_{\text{M,N}}$	= 1	EN 1992-4, Eq. (7.7)
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A _{c,N} [mm ²]	$A_{c,N}^0$ [mm ²]	c _{cr,N} [mm]	s _{cr,N} [mm]	f _{c,cyl} [N/mm ²]		
792,100	360,000	300.0	600.0	40.00		
e _{c1,N} [mm]	$\Psi_{\text{ec1,N}}$	e _{c2,N} [mm]	$\psi_{\text{ ec2,N}}$	$\psi_{s,N}$	$\psi_{\text{re},\text{N}}$	
0.0	1.000	25.0	0.923	1.000	1.000	
z [mm]	$\psi_{\text{M,N}}$	k ₁	$N_{Rk,c}^0$ [kN]	$\gamma_{M,c}$	N _{Rd,c} [kN]	N _{Ed} [kN]
0.0	1.000	7.700	137.742	1.500	186.505	20.000

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4 Shear load (EN 1992-4, Section 7.2.2)

	Load [kN]	Capacity [kN]	Utilization β_{V} [%]	Status
Steel failure (without lever arm)*	8.278	55.000	16	OK
Steel failure (with lever arm)*	8.278	9.009	92	OK
Pryout failure**	58.310	367.260	16	OK
Concrete edge failure in direction **	N/A	N/A	N/A	N/A

^{*} highest loaded anchor **anchor group (relevant anchors)

4.1 Steel failure (without lever arm)

4.2 Steel failure (with lever arm)

I [mm]

$$\begin{split} & V_{Ed} \leq V_{Rd,s,M} = \frac{V_{Rk,s,M}}{\gamma_{M,s}} & EN \ 1992\text{-}4, \ Table \ 7.2 \\ & V_{Rk,s,M} = \frac{\alpha_{M} \cdot M_{Rk,s}}{I_{a}} & EN \ 1992\text{-}4, \ Eq. \ 7.37 \\ & M_{Rk,s} = M_{Rk,s}^{0} \cdot \left(1 - \frac{N_{Ed}}{N_{Rd,s}}\right) & EN \ 1992\text{-}4, \ Eq. \ 7.38 \\ & I_{a} = e_{c} + \frac{t}{2} + a_{3} & EN \ 1992\text{-}4, \ Eq. \ 6.2 \end{split}$$

 α_{M}

62.5	2.00				
N_{Ed} / $N_{Rd,s}$	$1 - N_{Ed} / N_{Rd,s}$	$M_{Rk,s}^0$ [kNm]	$M_{Rk,s} = M_{Rk,s}^{0} (1 -$	$N_{Ed}/N_{Rd,s})$ [kNm]	
0.034	0.966	0.454	0.4	39	
$V_{Rk,s}^{M} = \alpha_{M}^{*}$	M _{Rk,s} / I [kN]	γ_{Ms}	$V_{Rd,s}^{M}$ [kN]	V _{Ed} [kN]	
14.053		1.560	9.009	8.278	



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4.3 Pryout failure (concrete cone relevant)

$V_{Ed} \le V_{Rd}$	$_{\text{l,cp}} = \frac{V_{\text{Rk,cp}}}{\gamma_{\text{M,c,p}}}$	EN 1992-4, Table 7.2
	$= k_8 \cdot \min \{N_{Rk,c}; N_{Rk,p}\}$	EN 1992-4, Eq. (7.39c)
$N_{Rk,c}$	$= N_{Rk,c}^{0} \cdot \frac{A_{c,N}}{A_{c,N}^{0}} \cdot \psi_{s,N} \cdot \psi_{re,N} \cdot \psi_{ec1,N} \cdot \psi_{ec2,N} \cdot \psi_{M,N}$	EN 1992-4, Eq. (7.1)
$egin{aligned} N_{Rk,c}^0 \ A_{c,N}^0 \end{aligned}$	$= k_1 \cdot \sqrt{f_{ck}} \cdot h_{ef}^{1,5}$	EN 1992-4, Eq. (7.2)
$A_{c,N}^0$	$= s_{cr,N} \cdot s_{cr,N}$	EN 1992-4, Eq. (7.3)
$\psi_{\text{ s,N}}$	$= 0.7 + 0.3 \cdot \frac{c}{c_{cr,N}} \le 1.00$	EN 1992-4, Eq. (7.4)
$\Psi_{\text{ ec1,N}}$	$= \frac{1}{1 + \left(\frac{2 \cdot e_{V,1}}{s_{cr,N}}\right)} \le 1.00$	EN 1992-4, Eq. (7.6)
$\Psi_{\text{ec2,N}}$	$= \frac{1}{1 + \left(\frac{2 \cdot e_{V,2}}{s_{cr,N}}\right)} \le 1.00$	EN 1992-4, Eq. (7.6)
$\psi_{\text{ M,N}}$	= 1	EN 1992-4, Eq. (7.7)

$A_{c,N}$ [mm ²]	$A_{c,N}^0$ [mm ²]	c _{cr,N} [mm]	s _{cr,N} [mm]	k ₈	f _{c,cyl} [N/mm ²]	
792,100	360,000	300.0	600.0	2.000	40.00	
e _{c1,V} [mm]	$\Psi_{\text{ ec1,N}}$	e _{c2,V} [mm]	$\Psi_{\text{ ec2,N}}$	$\psi_{\text{s,N}}$	$\psi_{\text{re},\text{N}}$	$\psi_{M,N}$
11.0	0.965	18.4	0.942	1.000	1.000	1.000
\mathbf{k}_{1}	N _{Rk,c} [kN]	$\gamma_{M,c,p}$	V _{Rd,cp} [kN]	V _{Ed} [kN]		
7.700	137.742	1.500	367.260	58.310	_	

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5 Combined tension and shear loads (EN 1992-4, Section 7.2.3)

Steel failure

β_{N}	β_{V}	α	Utilization $\beta_{N,V}$ [%]	Status	
0.034	0.151	2.000	3	OK	

$$\beta_N^{\alpha} + \beta_V^{\alpha} \le 1.0$$

Concrete failure

β_{N}	β_{V}	α	Utilization $\beta_{N,V}$ [%]	Status	
0.107	0.159	1.500	10	OK	

$$\beta_N^{\alpha} + \beta_V^{\alpha} \le 1.0$$

6 Displacements (highest loaded anchor)

Short term loading:

 N_{Sk} 2.278 [kN] 0.0127 [mm] 6.132 [kN] 0.2453 [mm] $V_{\rm Sk}$ δ_{V} 0.2456 [mm] Long term loading: 2.278 [kN] 0.0290 [mm] δ_{V} V_{Sk} = 6.132 [kN] 0.3679 [mm] 0.3691 [mm]

Comments: Tension displacements are valid with half of the required installation torque moment for uncracked concrete! Shear displacements are valid without friction between the concrete and the baseplate! The gap due to the drilled hole and clearance hole tolerances are not included in this calculation!

The acceptable anchor displacements depend on the fastened construction and must be defined by the designer!



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7 Warnings

- The anchor design methods in PROFIS Engineering require rigid baseplates per current regulations (AS 5216:2018, ETAG 001/Annex C, EOTA TR029 etc.). This means load re-distribution on the anchors due to elastic deformations of the baseplate are not considered the baseplate is assumed to be sufficiently stiff, in order not to be deformed when subjected to the design loading. PROFIS Engineering calculates the minimum required baseplate thickness with CBFEM to limit the stress of the baseplate based on the assumptions explained above. The proof if the rigid baseplate assumption is valid is not carried out by PROFIS Engineering. Input data and results must be checked for agreement with the existing conditions and for plausibility!
- · Checking the transfer of loads into the base material is required in accordance with EN 1992-4, Annex A!
- The design is only valid if the clearance hole in the fixture is not larger than the value given in Table 6.1 of EN 1992-4! For larger diameters of the clearance hole see section 6.2.2 of EN 1992-4!
- The accessory list in this report is for the information of the user only. In any case, the instructions for use provided with the product have to be followed to ensure a proper installation.
- For the determination of the Ψ_{re.ν} (concrete edge failure) a concrete cover of the edge reinforcement c = 30 mm is assumed
- Drilled hole cleaning must be performed according to instructions for use (blow twice with oil-free compressed air (min. 6 bar), brush twice, blow twice with oil-free compressed air (min. 6 bar)).
- · Characteristic bond resistances depend on short- and long-term temperatures.
- · Edge reinforcement is not required to avoid splitting failure
- The characteristic bond resistances depend on the return period (service life in years): 50

Fastening meets the design criteria!



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8 Installation data

Baseplate, steel: S 275; E = 210,000.00 N/mm 2 ; f_{yk} = 275.00 N/mm 2 Profile: Square hollow, 200 x 200 x 10,0; (L x W x T) = 200.0 mm x 200.0 mm x 10.0 mm

Hole diameter in the fixture: $d_f = 22.0 \text{ mm}$

Plate thickness (input): 35.0 mm

Recommended plate thickness: not calculated

Drilling method: Hammer drilled

Cleaning: Compressed air cleaning of the drilled hole according to instructions

for use is required

Anchor type and size: HIT-HY 200-A + HAS-U A4 M20 Item number: 2223929 HAS-U A4 M20x350 (insert) /

2022696 HIT-HY 200-A (mortar) Installation torque: 150 Nm

Hole diameter in the base material: 22.0 mm Hole depth in the base material: 200.0 mm

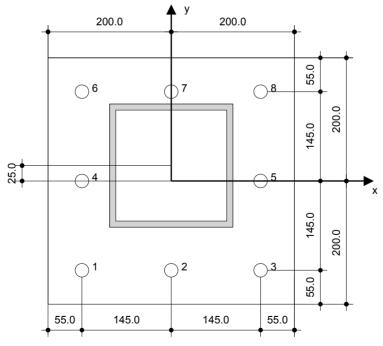
Minimum thickness of the base material: 244.0 mm

Hilti HAS-U threaded rod with HIT-HY 200 injection mortar with 200 mm embedment h_ef, M20, Stainless steel, SAFEset Auto cleaning Hammer drilling installation per ETA 11/0493

8.1 Recommended accessories

Drilling Cleaning Setting

- Suitable Rotary Hammer
- · Properly sized drill bit
- Compressed air with required accessories to blow from the bottom of the hole
- · Proper diameter wire brush
- · Dispenser including cassette and mixer
- Torque wrench



Coordinates Anchor mm

Anchor	x	у	C _{-x}	C+x	C _{-y}	C _{+y}	Anchor	X	у	C _{-x}	C+x	C _{-y}	C _{+y}
1	-145.0	-145.0	-	-	-	-	5	145.0	0.0	-	-	-	-
2	0.0	-145.0	-	-	-	-	6	-145.0	145.0	-	-	-	-
3	145.0	-145.0	-	-	-	-	7	0.0	145.0	-	-	-	-
4	-145.0	0.0	-	-	-	-	8	145.0	145.0	-	-	-	-

Input data and results must be checked for conformity with the existing conditions and for plausibility! PROFIS Engineering (c) 2003-2019 Hilti AG, FL-9494 Schaan Hilti is a registered Trademark of Hilti AG, Schaan



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9 Alternative fastening

9.1 Alternative fastening data

Anchor type and size: **HVU2 + HAS-U A4 M20**

Return period (service life in years):

Item number: 2223928 HAS-U A4 M20x300 (insert) / 2164509

HVU2 M20x170 (capsule)

Effective embedment depth: $h_{ef,act}$ = 170.0 mm, h_{nom} = 170.0 mm

Material:

Approval No .: ETA-16/0515 Issued I Valid: 13/11/2019 | -

Proof: Design Method EN 1992-4, Chemical

Stand-off installation: without clamping (anchor); restraint level (baseplate): 2.00; e_b = 35.0 mm; t = 35.0 mm

Hilti Grout: , multipurpose, f_{c,Grout} = 30.00 N/mm²

I_x x I_y x t = 400.0 mm x 400.0 mm x 35.0 mm; (Recommended plate thickness: not calculated) Baseplate^R:

Profile: Square hollow, 200 x 200 x 10,0; (L x W x T) = 200.0 mm x 200.0 mm x 10.0 mm

Base material: cracked concrete, C40/50, $f_{c,cyl}$ = 40.00 N/mm 2 ; h = 365.0 mm, Temp. short/long: 0/0 $^{\circ}$ C

Installation: hammer drilled hole, Installation condition: Dry

Reinforcement: No reinforcement or Reinforcement spacing >= 150 mm (any \emptyset) or >= 100 mm (\emptyset <= 10 mm)

no longitudinal edge reinforcement

Max. Utilisation with HVU2 + HAS-U A4 M20: 92 % Fastening meets the design criteria!

9.2 Installation data

Baseplate, steel: S 275; E = 210,000.00 N/mm²; f_{yk} = 275.00 N/mm²

Profile: Square hollow, 200 x 200 x 10,0; (L x W x T) = 200.0 mm x 200.0 mm x

10 0 mm

2164509 HVU2 M20x170 (capsule)

Hole diameter in the fixture: $d_f = 22.0 \text{ mm}$ Installation torque: 150 Nm

Plate thickness (input): 35.0 mm Hole diameter in the base material: 22.0 mm Recommended plate thickness: not calculated Hole depth in the base material: 170.0 mm

Drilling method: Hammer drilled Minimum thickness of the base material: 220.0 mm

Cleaning: Compressed air cleaning of the drilled hole according to instructions

for use is required

Hilti HAS-U threaded rod with HVU2 capsule mortar with 170 mm embedment h_ef, M20, Stainless steel, SAFEset Auto cleaning Hammer drilling installation per ETA-16/0515

9.2.1 Recommended accessories

Drilling Cleaning Setting · Compressed air with required

· Suitable Rotary Hammer

· Properly sized drill bit accessories to blow from the bottom of

· Proper diameter wire brush

· HVA square drive shafts

· Torque wrench

Anchor type and size: HVU2 + HAS-U A4 M20

Item number: 2223928 HAS-U A4 M20x300 (insert) /

Input data and results must be checked for conformity with the existing conditions and for plausibility! PROFIS Engineering (c) 2003-2019 Hilti AG, FL-9494 Schaan Hilti is a registered Trademark of Hilti AG, Schaan 11



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10 Remarks; Your Cooperation Duties

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 the regular backup of programs and data and, if applicable, carry out the updates of the Software offered by Hilti on a regular basis. If you do
 not use the AutoUpdate function of the Software, you must ensure that you are using the current and thus up-to-date version of the Software
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