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Company:
 Address:
 Phone | Fax: |
 Design: Connection between guard post base and slab
 Fastening point:

Page: 1
 Specifier:
 E-Mail:
 Date: 9/20/2022

Specifier's comments:

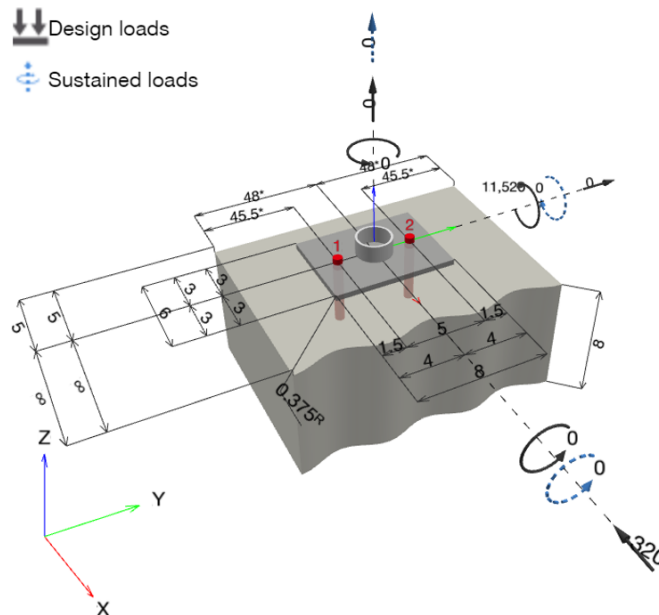
1 Input data



| | |
|----------------------------------|--|
| Anchor type and diameter: | HIT-HY 200 V3 + HIT-Z 5/8 |
| Item number: | 2018446 HIT-Z 5/8" x 6" (element) / 2334276 HIT-HY 200-R V3 (adhesive) |
| Effective embedment depth: | $h_{ef,act} = 4.000$ in. ($h_{ef,limit} = -$ in.) |
| Material: | DIN EN ISO 4042 |
| Evaluation Service Report: | ESR-4868 |
| Issued Valid: | 11/1/2021 11/1/2022 |
| Proof: | Design Method ACI 318-19 / Chem |
| Stand-off installation: | $e_b = 0.000$ in. (no stand-off); $t = 0.375$ in. |
| Anchor plate ^R : | $l_x \times l_y \times t = 6.000$ in. x 8.000 in. x 0.375 in.; (Recommended plate thickness: not calculated) |
| Profile: | Steel pipe, PIPE2STD; (L x W x T) = 2.380 in. x 2.380 in. x 0.154 in. |
| Base material: | cracked concrete, 3000, $f'_c = 3,000$ psi; $h = 8.000$ in., Temp. short/long: 75/75 °F |
| Installation: | hammer drilled hole, Installation condition: Dry |
| Reinforcement: | tension: not present, shear: not present; no supplemental splitting reinforcement present edge reinforcement: none or < No. 4 bar |

^R - The anchor calculation is based on a rigid anchor plate assumption.

Geometry [in.] & Loading [lb, in.lb]



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1.1 Design results

| Case | Description | Forces [lb] / Moments [in.lb] | Seismic | Max. Util. Anchor [%] |
|------|---------------------|--|---------|-----------------------|
| 1 | 1.2DL + 1.6L + 1.0W | N = 0; V _x = -320; V _y = 0; M _x = 0; M _y = -11,520; M _z = 0; | no | 74 |

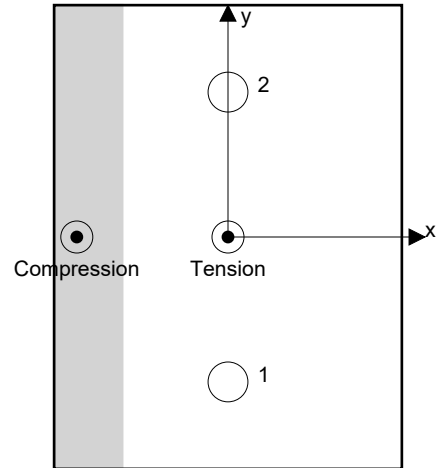
2 Load case/Resulting anchor forces

Anchor reactions [lb]

Tension force: (+Tension, -Compression)

| Anchor | Tension force | Shear force | Shear force x | Shear force y |
|--------|---------------|-------------|---------------|---------------|
| 1 | 2,208 | 160 | -160 | 0 |
| 2 | 2,208 | 160 | -160 | 0 |

max. concrete compressive strain: 0.22 [‰]
 max. concrete compressive stress: 941 [psi]
 resulting tension force in (x/y)=(0.000/-0.000): 4,416 [lb]
 resulting compression force in (x/y)=(-2.609/-0.000): 4,416 [lb]



Anchor forces are calculated based on the assumption of a rigid anchor plate.

3 Tension load

| | Load N _{ua} [lb] | Capacity ϕN_n [lb] | Utilization $\beta_N = N_{ua} / \phi N_n$ | Status |
|---------------------------------------|---------------------------|--------------------------|---|--------|
| Steel Strength* | 2,208 | 13,849 | 16 | OK |
| Pullout Strength* | 2,208 | 13,904 | 16 | OK |
| Sustained Tension Load Bond Strength* | N/A | N/A | N/A | N/A |
| Concrete Breakout Failure** | 4,416 | 5,973 | 74 | OK |

* highest loaded anchor **anchor group (anchors in tension)



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3.1 Steel Strength

N_{sa} = ESR value refer to ICC-ES ESR-4868
 $\phi N_{sa} \geq N_{ua}$ ACI 318-19 Table 17.5.2

Variables

| | |
|--------------------------------|-----------------|
| $A_{se,N}$ [in. ²] | f_{uta} [psi] |
| 0.23 | 94,200 |

Calculations

| |
|---------------|
| N_{sa} [lb] |
| 21,306 |

Results

| | | | |
|---------------|----------------|--------------------|---------------|
| N_{sa} [lb] | ϕ_{steel} | ϕN_{sa} [lb] | N_{ua} [lb] |
| 21,306 | 0.650 | 13,849 | 2,208 |

3.2 Pullout Strength

$N_{pn} = N_p \lambda_a$ refer to ICC-ES ESR-4868
 $\phi N_{pn} \geq N_{ua}$ ACI 318-19 Table 17.5.2

Variables

| | |
|-------------|------------|
| λ_a | N_p [lb] |
| 1.000 | 21,391 |

Calculations

| |
|---------------|
| N_{pn} [lb] |
| 21,391 |

Results

| | | | |
|---------------|-------------------|--------------------|---------------|
| N_{pn} [lb] | $\phi_{concrete}$ | ϕN_{pn} [lb] | N_{ua} [lb] |
| 21,391 | 0.650 | 13,904 | 2,208 |



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3.3 Concrete Breakout Failure

$$N_{cbg} = \left(\frac{A_{Nc}}{A_{Nc0}} \right) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \quad \text{ACI 318-19 Eq. (17.6.2.1b)}$$

$$\phi N_{cbg} \geq N_{ua} \quad \text{ACI 318-19 Table 17.5.2}$$

A_{Nc} see ACI 318-19, Section 17.6.2.1, Fig. R 17.6.2.1(b)

$$A_{Nc0} = 9 h_{ef}^2 \quad \text{ACI 318-19 Eq. (17.6.2.1.4)}$$

$$\psi_{ec,N} = \left(\frac{1}{1 + \frac{2 e_N}{3 h_{ef}}} \right) \leq 1.0 \quad \text{ACI 318-19 Eq. (17.6.2.3.1)}$$

$$\psi_{ed,N} = 0.7 + 0.3 \left(\frac{c_{a,min}}{1.5 h_{ef}} \right) \leq 1.0 \quad \text{ACI 318-19 Eq. (17.6.2.4.1b)}$$

$$\psi_{cp,N} = \text{MAX} \left(\frac{c_{a,min}}{c_{ac}}, \frac{1.5 h_{ef}}{c_{ac}} \right) \leq 1.0 \quad \text{ACI 318-19 Eq. (17.6.2.6.1b)}$$

$$N_b = k_c \lambda_a \sqrt{f'_c} h_{ef}^{1.5} \quad \text{ACI 318-19 Eq. (17.6.2.2.1)}$$

Variables

| | | | | |
|----------------|------------------|------------------|-------------------|--------------|
| h_{ef} [in.] | $e_{c1,N}$ [in.] | $e_{c2,N}$ [in.] | $c_{a,min}$ [in.] | $\psi_{c,N}$ |
| 4.000 | 0.000 | 0.000 | 5.000 | 1.000 |
| c_{ac} [in.] | k_c | λ_a | f'_c [psij] | |
| 8.800 | 17 | 1.000 | 3,000 | |

Calculations

| | | | | | | |
|------------------------------|-------------------------------|----------------|----------------|---------------|---------------|------------|
| A_{Nc} [in. ²] | A_{Nc0} [in. ²] | $\psi_{ec1,N}$ | $\psi_{ec2,N}$ | $\psi_{ed,N}$ | $\psi_{cp,N}$ | N_b [lb] |
| 187.00 | 144.00 | 1.000 | 1.000 | 0.950 | 1.000 | 7,449 |

Results

| | | | |
|----------------|-------------------|---------------------|---------------|
| N_{cbg} [lb] | $\phi_{concrete}$ | ϕN_{cbg} [lb] | N_{ua} [lb] |
| 9,190 | 0.650 | 5,973 | 4,416 |



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4 Shear load

| | Load V_{ua} [lb] | Capacity ϕV_n [lb] | Utilization $\beta_v = V_{ua} / \phi V_n$ | Status |
|---|--------------------|--------------------------|---|--------|
| Steel Strength* | 160 | 5,625 | 3 | OK |
| Steel failure (with lever arm)* | N/A | N/A | N/A | N/A |
| Pryout Strength (Concrete Breakout Strength controls)** | 320 | 12,865 | 3 | OK |
| Concrete edge failure in direction x-** | 320 | 4,585 | 7 | OK |

* highest loaded anchor **anchor group (relevant anchors)

4.1 Steel Strength

V_{sa} = ESR value refer to ICC-ES ESR-4868
 $\phi V_{steel} \geq V_{ua}$ ACI 318-19 Table 17.5.2

Variables

| $A_{se,v}$ [in. ²] | f_{uta} [psi] |
|--------------------------------|-----------------|
| 0.23 | 94,200 |

Calculations

| V_{sa} [lb] |
|---------------|
| 9,375 |

Results

| V_{sa} [lb] | ϕ_{steel} | ϕV_{sa} [lb] | V_{ua} [lb] |
|---------------|----------------|--------------------|---------------|
| 9,375 | 0.600 | 5,625 | 160 |



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4.2 Pryout Strength (Concrete Breakout Strength controls)

$$V_{cp,g} = k_{cp} \left[\left(\frac{A_{Nc}}{A_{Nc0}} \right) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \right] \quad \text{ACI 318-19 Eq. (17.7.3.1b)}$$

$$\phi V_{cp,g} \geq V_{ua} \quad \text{ACI 318-19 Table 17.5.2}$$

A_{Nc} see ACI 318-19, Section 17.6.2.1, Fig. R 17.6.2.1(b)

$$A_{Nc0} = 9 h_{ef}^2 \quad \text{ACI 318-19 Eq. (17.6.2.1.4)}$$

$$\psi_{ec,N} = \left(\frac{1}{1 + \frac{2 e_N}{3 h_{ef}}} \right) \leq 1.0 \quad \text{ACI 318-19 Eq. (17.6.2.3.1)}$$

$$\psi_{ed,N} = 0.7 + 0.3 \left(\frac{c_{a,min}}{1.5 h_{ef}} \right) \leq 1.0 \quad \text{ACI 318-19 Eq. (17.6.2.4.1b)}$$

$$\psi_{cp,N} = \text{MAX} \left(\frac{c_{a,min}}{c_{ac}}, \frac{1.5 h_{ef}}{c_{ac}} \right) \leq 1.0 \quad \text{ACI 318-19 Eq. (17.6.2.6.1b)}$$

$$N_b = k_c \lambda_a \sqrt{f'_c} h_{ef}^{1.5} \quad \text{ACI 318-19 Eq. (17.6.2.2.1)}$$

Variables

| | | | | |
|--------------|----------------|------------------|------------------|-------------------|
| k_{cp} | h_{ef} [in.] | $e_{c1,N}$ [in.] | $e_{c2,N}$ [in.] | $c_{a,min}$ [in.] |
| 2 | 4.000 | 0.000 | 0.000 | 5.000 |
| $\psi_{c,N}$ | c_{ac} [in.] | k_c | λ_a | f'_c [psi] |
| 1.000 | 8.800 | 17 | 1.000 | 3,000 |

Calculations

| | | | | | | |
|------------------------------|-------------------------------|----------------|----------------|---------------|---------------|------------|
| A_{Nc} [in. ²] | A_{Nc0} [in. ²] | $\psi_{ec1,N}$ | $\psi_{ec2,N}$ | $\psi_{ed,N}$ | $\psi_{cp,N}$ | N_b [lb] |
| 187.00 | 144.00 | 1.000 | 1.000 | 0.950 | 1.000 | 7,449 |

Results

| | | | |
|-----------------|-------------------|----------------------|---------------|
| $V_{cp,g}$ [lb] | $\phi_{concrete}$ | $\phi V_{cp,g}$ [lb] | V_{ua} [lb] |
| 18,379 | 0.700 | 12,865 | 320 |



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4.3 Concrete edge failure in direction x-

$$V_{cbg} = \left(\frac{A_{Vc}}{A_{Vc0}} \right) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} \Psi_{parallel,V} V_b \quad \text{ACI 318-19 Eq. (17.7.2.1b)}$$

$$\phi V_{cbg} \geq V_{ua} \quad \text{ACI 318-19 Table 17.5.2}$$

A_{Vc} see ACI 318-19, Section 17.7.2.1, Fig. R 17.7.2.1(b)

$$A_{Vc0} = 4.5 c_{a1}^2 \quad \text{ACI 318-19 Eq. (17.7.2.1.3)}$$

$$\Psi_{ec,V} = \left(\frac{1}{1 + \frac{e_v}{1.5c_{a1}}} \right) \leq 1.0 \quad \text{ACI 318-19 Eq. (17.7.2.3.1)}$$

$$\Psi_{ed,V} = 0.7 + 0.3 \left(\frac{c_{a2}}{1.5c_{a1}} \right) \leq 1.0 \quad \text{ACI 318-19 Eq. (17.7.2.4.1b)}$$

$$\Psi_{h,V} = \sqrt{\frac{1.5c_{a1}}{h_a}} \geq 1.0 \quad \text{ACI 318-19 Eq. (17.7.2.6.1)}$$

$$V_b = \left(7 \left(\frac{l_e}{d_a} \right)^{0.2} \sqrt{d_a} \right) \lambda_a \sqrt{f'_c} c_{a1}^{1.5} \quad \text{ACI 318-19 Eq. (17.7.2.2.1a)}$$

Variables

| | | | | |
|----------------|----------------|----------------|--------------|---------------------|
| c_{a1} [in.] | c_{a2} [in.] | e_{cV} [in.] | $\Psi_{c,V}$ | h_a [in.] |
| 5.000 | 45.500 | 0.000 | 1.000 | 8.000 |
| l_e [in.] | λ_a | d_a [in.] | f'_c [psi] | $\Psi_{parallel,V}$ |
| 4.000 | 1.000 | 0.625 | 3,000 | 1.000 |

Calculations

| | | | | | |
|------------------------------|-------------------------------|---------------|---------------|--------------|------------|
| A_{Vc} [in. ²] | A_{Vc0} [in. ²] | $\Psi_{ec,V}$ | $\Psi_{ed,V}$ | $\Psi_{h,V}$ | V_b [lb] |
| 150.00 | 112.50 | 1.000 | 1.000 | 1.000 | 4,912 |

Results

| | | | |
|----------------|-------------------|---------------------|---------------|
| V_{cbg} [lb] | $\phi_{concrete}$ | ϕV_{cbg} [lb] | V_{ua} [lb] |
| 6,550 | 0.700 | 4,585 | 320 |

5 Combined tension and shear loads, per ACI 318-19 section 17.8

| | | | | |
|-----------|-----------|---------|-------------------------------|--------|
| β_N | β_V | ζ | Utilization $\beta_{N,V}$ [%] | Status |
| 0.739 | 0.070 | 5/3 | 62 | OK |

$$\beta_{NV} = \beta_N^{\zeta} + \beta_V^{\zeta} \leq 1$$



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6 Warnings

- The anchor design methods in PROFIS Engineering require rigid anchor plates per current regulations (AS 5216:2021, ETAG 001/Annex C, EOTA TR029 etc.). This means load re-distribution on the anchors due to elastic deformations of the anchor plate are not considered - the anchor plate is assumed to be sufficiently stiff, in order not to be deformed when subjected to the design loading. PROFIS Engineering calculates the minimum required anchor plate thickness with CBFEM to limit the stress of the anchor plate based on the assumptions explained above. The proof if the rigid anchor plate assumption is valid is not carried out by PROFIS Engineering. Input data and results must be checked for agreement with the existing conditions and for plausibility!
- Condition A applies where the potential concrete failure surfaces are crossed by supplementary reinforcement proportioned to tie the potential concrete failure prism into the structural member. Condition B applies where such supplementary reinforcement is not provided, or where pullout or pryout strength governs.
- Design Strengths of adhesive anchor systems are influenced by the cleaning method. Refer to the INSTRUCTIONS FOR USE given in the Evaluation Service Report for cleaning and installation instructions.
- For additional information about ACI 318 strength design provisions, please go to <https://submittals.us.hilti.com/PROFISAnchorDesignGuide/>
- Installation of Hilti adhesive anchor systems shall be performed by personnel trained to install Hilti adhesive anchors. Reference ACI 318-19, Section 26.7.

Fastening meets the design criteria!

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7 Installation data

Profile: Steel pipe, PIPE2STD; (L x W x T) = 2.380 in. x 2.380 in. x 0.154 in.

Hole diameter in the fixture (pre-setting) : $d_f = 0.687$ in.

Hole diameter in the fixture (through fastening) : $d_f = 0.812$ in.

Plate thickness (input): 0.375 in.

Recommended plate thickness: not calculated

Drilling method: Hammer drilled

Cleaning: Compressed air cleaning of the drilled hole according to instructions for use is required

Anchor type and diameter: HIT-HY 200 V3 + HIT-Z 5/8
 Item number: 2018446 HIT-Z 5/8" x 6" (element) / 2334276
 HIT-HY 200-R V3 (adhesive)

Maximum installation torque: 708 in.lb

Hole diameter in the base material: 0.750 in.

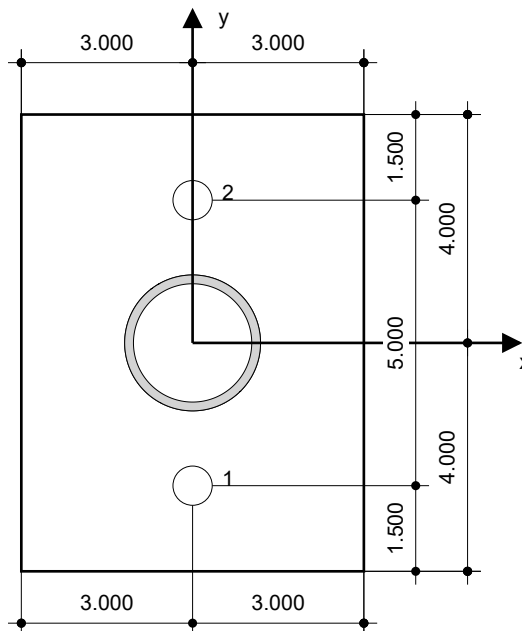
Hole depth in the base material: 4.000 in.

Minimum thickness of the base material: 8.000 in.

5/8 Hilti HIT-Z Carbon steel non-cleaning bonded expansion anchor with Hilti HIT-HY 200 V3 Safe Set System

7.1 Recommended accessories

| Drilling | Cleaning | Setting |
|--|---|---|
| <ul style="list-style-type: none"> Suitable Rotary Hammer Properly sized drill bit | <ul style="list-style-type: none"> - | <ul style="list-style-type: none"> Dispenser including cassette and mixer Torque wrench |



Coordinates Anchor [in.]

| Anchor | x | y | c _{-x} | c _{+x} | c _{-y} | c _{+y} |
|--------|-------|--------|-----------------|-----------------|-----------------|-----------------|
| 1 | 0.000 | -2.500 | 5.000 | - | 45.500 | 50.500 |
| 2 | 0.000 | 2.500 | 5.000 | - | 50.500 | 45.500 |



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8 Remarks; Your Cooperation Duties

- Any and all information and data contained in the Software concern solely the use of Hilti products and are based on the principles, formulas and security regulations in accordance with Hilti's technical directions and operating, mounting and assembly instructions, etc., that must be strictly complied with by the user. All figures contained therein are average figures, and therefore use-specific tests are to be conducted prior to using the relevant Hilti product. The results of the calculations carried out by means of the Software are based essentially on the data you put in. Therefore, you bear the sole responsibility for the absence of errors, the completeness and the relevance of the data to be put in by you. Moreover, you bear sole responsibility for having the results of the calculation checked and cleared by an expert, particularly with regard to compliance with applicable norms and permits, prior to using them for your specific facility. The Software serves only as an aid to interpret norms and permits without any guarantee as to the absence of errors, the correctness and the relevance of the results or suitability for a specific application.
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